SOIL PROPERTIES AND THEIR CORRELATIONS SECOND EDITION



MICHAEL CARTER STEPHEN P. BENTLEY



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SECOND EDITION

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Preface

The aims of this book are to provide a summary and discussion of commonly used soil engineering properties and to give correlations of various engineering properties.

The book includes:

- a compendium of published correlations;
- discussions of the reliability, accuracy and usefulness of the various correlations;
- practical advice on how soil properties are used in the assessment and design of geotechnical problems, including basic concepts, and limitations on their use that need to be considered; and
- descriptions of the measurement of soil properties, and how results are affected by the method of measurement and the expertise of technicians carrying out the testing.

A consideration in describing the various properties has been an awareness by the authors that many geotechnical engineers and engineering geologists have little, if any, hands-on experience of laboratory testing, and are often unaware of the procedures used to obtain the various soil properties and of the effects of poor or inappropriate practice.

The properties are also described in relation to their use in geotechnical analysis, in a way that we hope will give students and younger engineers an in-depth appreciation of the appropriate use of each property and the pitfalls to avoid, and should also provide a useful reminder to more experienced professionals. Many soil correlations were established in the early decades of soil mechanics, with there being no need to repeat the work once correlations had been established and verified by sufficient researchers. As a consequence, the correlations given in this book span a wide range of time, a few as far back as the 1930s, but we have also presented more recent work where this adds useful information. However, our intention in selecting correlations is to present those that will be of wide practical application, and the book is not intended as a research review. To aid their use in spreadsheet calculations, we have derived mathematical expressions to fit many of the correlations that were originally given only graphically. We have also tried to keep the work independent of national design codes, but it inevitably contains references to practices that are more prevalent in the English-speaking world. Where references are made to classification systems and associated codes we have, where possible, included references to both UK and US practice.

We envisage and recommend that correlations be used in two ways: firstly, to obtain values of a property that has not been measured; and secondly, to provide additional values where some direct measurements of the property have been made. In the first case, where no values of a particular property have been directly measured, the values obtained from correlations should be viewed with caution and treated as preliminary, especially where the property value is critical to the predicted performance of a design. Where correlations are used in combination with direct measurements to provide supplementary values, the accuracy and reliability of the correlations can usually be verified, fine-tuning the correlation if necessary, which may allow the values obtained by correlation to be viewed with more confidence.

While every care has been taken in the preparation of this book, with the very large amount of information that has been assembled it is possible that some errors have occurred; users should satisfy themselves that the information presented is correct. The authors can take no responsibility for consequences resulting from any errors in the book. The views expressed about the reliability and accuracy of correlations, typical values and other published information are based on the authors' own experience and may not accord with those of other geotechnical specialists.

Acknowledgements

In creating a compendium of published correlations, we had to seek permission from many authors around the globe; for her role in this important and painstaking task, the authors would like to thank Carol Clark. Bringing together such a large number of disparate items of information from many sources also involved a great deal of checking, and our thanks go to ex-colleagues Jason Williams and Max Lundie for their checking of some of the work, and especially to Mark Campbell who read through the entire script, noting errors and giving many helpful suggestions.

List of Symbols

Symbol	Name of variable	Typical units (SI)*
α	A scaling factor for estimating footing settlements from plate bearing test results.	D
α	A factor for estimating values of coefficient of volume compressibility from static cone test results.	D
α	Adhesion factor, for pile calculations.	D
α	A factor used to estimate the pull-out resistance of a soil reinforcement grid.	D
Δp	A distance above or below the A-line on a standard plasticity chart.	%
θ	Angle of a plane, from the direction of maximum principle stress, on which stresses act.	Degrees
μ	Viscosity of permeant for general seepage calculations.	kN.s/m ²
ν	Poisson's ratio.	D
π	Ratio of the circumference of a circle to its diameter (≈ 3.14159).	D
ρ	Settlement.	m, mm
σ	Direct stress.	kPa (kN/m ²)
σ'	Effective direct stress.	kPa (kN/m ²)

$\sigma_1, \sigma_2, \sigma_3$	Maximum, intermediate and minimum principal stresses	kPa (kN/m ²)
$\sigma_{_n}$	Effective earth pressure, used in soil nail calculations.	kPa (kN/m ²)
σ_{v}, σ'_{v}	Vertical stress, or overburden pressure, in total and effective stress terms, respectively.	kPa (kN/m²)
τ	Shear stress.	kPa (kN/m ²)
γ	Bulk density of soil.	kN/m ³
γ,	Dry density of soil.	kN/m ³
γ_{dmax}	Maximum dry density, for relative density calculations.	kN/m ³
γ_{dmin}	Minimum dry density, for relative density calculations.	kN/m ³
γ_p	Density of permeant for general seepage equation.	kN/m ⁴
γ_{sub}	Submerged density of soil.	kN/m ³
γ_w	Density of water.	kN/m ³
φ	Angle of shearing resistance (general, or in total stress terms).	Degrees
arphi'	Effective stress angle of shearing resistance.	Degrees
$\varphi_{_d}$	Drained angle of shearing resistance.	Degrees
φ_r	Residual angle of shearing resistance (general).	Degrees
a	Air voids content of soil.	%
а	Component of influence factor I_c for estimating settlements of footings on sands.	D
Α	Area (nominal) of soil water flow.	m ²
Α	A correction factor for rod energy ratio in the standard penetration test.	D
Α	Percentage passing a 2.4 mm sieve, used in the calculation of suitability index.	%
Α	A constant used in the estimation of swelling potential from plasticity index.	D
A_{c}	Activity value (of a clay).	D
A_p^{c}	End area of penetration cone in a 1standard penetration test.	mm ²
A_{s}	End area of penetration cone in a dynamic probe.	mm ²

a_{v}	Coefficient of compressibility. (See also	m²/MN
L	m_{v} , coefficient of volume compressibility.)	D
D	Component of influence factor I_c for	D
	estimating settlements of footings on	
D	sallus.	
D D	A constant used in the estimation of	
D	A constant used in the estimation of	D
0	Shape feater in general seepage	D
С	shape factor in general seepage	D
	Calculations.	$1_{\rm r} \mathbf{D}_{\rm o} \left(1_{\rm r} \mathbf{N} / m^2 \right)$
с С	Collesion.	KPa (KIN/III ⁻)
C	the coloriation of activity for a close	D
/	Effective stress ashesist	$1 - D{2} \left(1 - N \frac{1}{2} - N \frac{1}{2} \right)$
c C	Effective stress conesion.	KPa (KN/m ²)
C_1	Constant used in Hazen's formula to	D
CDD	California Dearing Datis	01
CBK	Canfornia Bearing Ratio.	% D
C_{c}	Coefficient of curvature (coefficient of	D
C	grading).	D
C_{c}, C_{r}	compression index, recompression	D
_	index, respectively.	$1 - D_{2} \left(1 - N \frac{1}{2} - N \frac{1}{2} \right)$
C_d	Drained conesion.	$KPa (KIN/m^2)$
CI C	Consistency index.	% D
C_N	Correction factor for overburden	D
	pressure, applied to SPI <i>N</i> -values.	10 (1)1(2)
C_u	Undrained cohesion, shear strength.	$kPa (kN/m^2)$
C_{u}	Coefficient of uniformity.	D
C_{v}	Coefficient of consolidation.	cm ² /s, m ² /year
C_{α}	Secondary compression index.	$(\log_{10} \text{ time})^{-1}$
$C_{\alpha\varepsilon}, C'$	Modified secondary compression index	$(\log_{10} \text{time})^{-1}$
	(sometimes referred to simply as the	
1	secondary compression index).	
d	Maximum length of drainage path in	m
D	consolidation calculations.	
D	Depth of foundation (when calculating	m
D	allowable bearing pressures on sands).	
D_{10}	The 10% particle size, also called the	mm (or µm)
	effective size.	
D_{30}, D_{60}	The 30% and 60% particle sizes,	mm (or µm)
	respectively.	

D_n	The particle size at which $n\%$ of the	mm (or μ m)
מ	material is finer. See also D_{10} , D_{30} , D_{60} .	D
D_r	An effective perticle size for permechility	D
D_{s}	All effective particle size for permeability	111111
0	Voids ratio	D
e	The natural number approximately	D
e	2 718	D
F	2.710. Young's modulus (also called the elastic	kPa MPa
L	modulus).	KI a, IVII a
e_{1}, e_{2}	Initial and final voids ratios in	D
	consolidation testing.	
E_d	Deformation modulus (also called the	kPa, MPa
	constrained modulus).	
e_{max}	Maximum voids ratio, for relative	D
	density calculations.	
$e_{_{min}}$	Minimum voids ratio, for relative	D
	density calculations.	
ER_r	Rod energy ratio in standard penetration	D
	test.	
F	The percentage passing the 75 µm sieve,	%
	used in the calculation of AASHTO	
	classification group index.	
$f_p f_s, f_t$	Shape, layer thicknes and time factors,	D
	respectively, for estimating settlements	
	of footings on sands.	
F_p	Drop distance of monkey (falling	mm
	hammer)	
_	in a dynamic probe.	
F_{s}	Drop distance of monkey (falling	mm
~	hammer) in a standard penetration test.	
G	Shear modulus.	kPa, MPa
G_{s}	Specific gravity of soil solids	D
h	Thickness of specimen in consolidation	mm
	testing.	
Н	Thickness of a compressible layer in	m
	consolidation testing.	D
1	Hydraulic gradient in soil water flow.	D
I _c	Influence factor for estimation of	D
	settlements of footings on sands.	

I_r	Rigidity index, used in rate-of-	D
1	settlement estimates based on static	
	piezocone test results.	
I_r	Swell index, used in the estimation of	D
	swelling pressure.	
k	Coefficient of permeability.	m/s, m/year
Κ	A constant used in the estimation of	D
	swelling potential from plasticity index.	
K_{o}	Coefficient of earth pressure at rest.	D
K _d	Depth factor for allowable bearing	D
u	pressures on sands.	
K _s	Earth pressure coefficient use in driven	D
5	pile calculations.	
L	Footing length.	m
LI	Liquidity index.	%
LL	Liquid limit.	%
т	Moisture (water) content of soil.	%
M_{p}	Mass of monkey (falling hammer) in a	kg
P	dynamic probe.	
M _s	Mass of monkey (falling hammer) in a	kg
3	standard penetration test.	
m,	Coefficient of volume compressibility.	m²/MN
,	(See also a_v , coefficient of compressibility.)	
n	Porosity of soil.	D
n	A factor used to estimate undrained	D
	shear strength from consistency index	
	or liquidity index.	
Ν	SPT <i>N</i> -value; blows of standard hammer	Blows
	to drive the SPT sampler or cone	
	300 mm.	
N_1	SPT N-value corrected for overburden	Blows
	pressure.	
$N_{1(60)}$	SPT N-value corrected for overburden	Blows
()	pressure and to a rod energy ratio of	
	60%.	
N_{60}	SPT <i>N</i> -value corrected for rod energy	Blows
	ratio, ER_r . (the "60" refers to	
	standardisation to 60% rod energy.)	
N _{corrected}	SPT <i>N</i> -value corrected for silts and fine	Blows
	sands below the groundwater table.	

N_{k}	A factor used in the estimation of	D
	undrained shear strength from static	
	cone tip resistance.	
O_{40}, O_{80}	Pore diameters at which 40% and 80%	mm, µm
40 80	of the pores are finer	
OCR	Overconsolidation ratio.	D
р	Previous maximum overburden pressure,	D
-	used in estimating settlements of	
	footings on sands.	
p_{1}, p_{2}	Initial and final pressures used in a stage	kPa (kN/m ²)
· · · 2	of consolidation testing.	
PI	Plasticity index.	%
PL	Plastic limit.	%
PM	Plasticity modulus.	%
Р	Penetration for each blow count in a	mm
р	dynamic probe.	
Р	Penetration for each blow count in a	mm
S	standard penetration test.	
a	Ouantity of flow of water through soil	m ³ /s, m ³ /vear
1	per unit time.	,
q	Bearing pressure.	kPa (kN/m ²)
\overline{q}_{-}	Allowable bearing pressure.	MPa (MN/m^2)
a	Measured cone resistance (pressure) in	kPa (kN/m ²)
10	static cone tests.	
q_{\dots}	Ultimate bearing capacity.	kPa (kN/m ²)
\hat{R}^{u}	Component of influence factor f for	D
	estimating settlements of footings on	
	sands.	
S	Degree of saturation.	%
S	Swelling potential.	%
<i>s</i> , <i>s</i> ,	Undrained shear strength.	kPa (kN/m ²)
S.	Sensitivity	D
ŚĹ	Shrinkage limit.	%
t	Time, used in calculations or rates	s, years
	of consolidation and secondary	
	compression.	
t_{1}, t_{2}	Start and end times for secondary	s, years
1 2	compression calculations.	-
$T_{\rm u}$	Basic time factor, used in calculations or	D
V	rates of consolidation.	

и	Pore water pressure.	kPa (kN/m ²)
U	Degree of consolidation.	D
v	Nominal velocity of flow of water through soil.	m/s, m/year
<i>V</i> _t	True velocity of flow of water through soil.	m/s, m/year
$W_{_{LW}}$	Weighted liquid limit, used in the estimation of swelling potential.	%
$W_{_{W}}$	Weight of water (in the model soil sample).	g
Y	Rate of frost heave.	mm/day
* **		

*D=dimensionless; % values are also essentially dimensionless.

List of Property Values and Correlations in the Tables and Figures

	р	Index roperti	es	Der	sity		Compressibility			Stre	ngth	Specialised					Probe testing			
Table	Grading	Moisture content	Plasticity/ consistency limits	Density	Relative density	Permeability	Total settlement (m_{ν} , C_c and settlement of sands)	Rate of settlement (c _v)	Coefficients of secondary compression	Shear strength	California Bearing Ratio	Shrinkage and swelling characteristics	Frost susceptibility	Susceptibility to combustion	Stresses at soil-structure interfaces	Soil classification	Standard penetration test	Dynamic cone tests	Static cone tests	
3.1				~																
3.2				~												~				
3.3				~												~				
3.4					~												~			
3.5																	~			
3.6																	~			
3.7																	~			
4.1						~														
5.1							~									~				
5.2							~													
5.3								~								~				
5.4							~		~							~		-		
5.5							~									~			~	
6.1										~						~		-		
6.2			~							~										
6.3										~						~				
6.4					~					~						~		-		
6.5										~						~				
7.1											~									
7.2			~								~					~				
8.1												~								
8.2			~													-				
8.3												✓				-				
8.4			✓									✓ 								
8.5			×									×								
8.0	*		*									*								
0.7	*		*																	
9.1	•		•										•			./				
11.1													•		1	•				
11.2																				
A1																•				
A2																~				
A3																~				
A4																~				
A5																~				
A6																~				
A7																~				
A8																~				
B1																			1	

	Index properties			Density			Compressibility			Stre	ngth		Spec	ialised			Probe testing			
Figure	Grading	Moisture content	Plasticity/consistency limits	Density	Relative density	Permeability	Total settlement (m_{ν} , C_c and settlement of sands)	Rate of settlement (c _v)	Coefficients of secondary compression	Shear strength	California Bearing Ratio	Shrinkage and swelling characteristics	Frost susceptibility	Susceptibility to combustion	Stresses at soil-structure interfaces	Soil classification	Standard penetration test	Dynamic cone tests	Static cone tests	
1.13										~										
2.1	~															~				
2.2	~																			
2.4			~																	
2.5			~													~				
3.1				~																
3.2		~		~																
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	Index properties			Density			Com	pressi	bility	Stre	ngth		Spec	ialised			Probe testing			
Figure	Grading	Moisture content	Plasticity/consistency limits	Density	Relative density	Permeability	Total settlement (m_{ν} , C_c and settlement of sands)	Rate of settlement (c_v)	Coefficients of secondary compression	Shear strength	California Bearing Ratio	Shrinkage and swelling characteristics	Frost susceptibility	Susceptibility to combustion	Stresses at soil-structure interfaces	Soil classification	Standard penetration test	Dynamic cone tests	Static cone tests	
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